

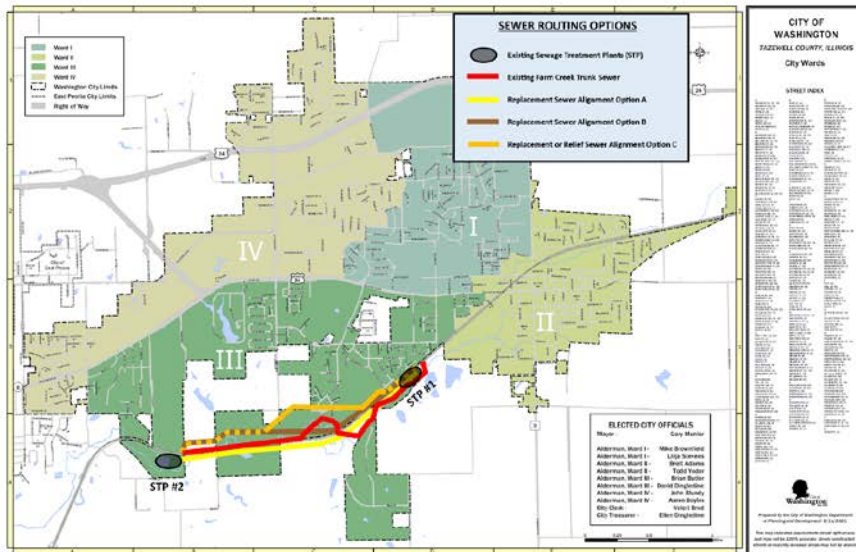


DRAFT

Certified DBE-WBE-BEP

City of Washington, Illinois
Farm Creek Trunk Sewer
3rd Party Alignment Analysis
HCE Job# 21911

February 15, 2022



Prepared By: Howard J. Hamilton, P.E., CFM, CPESC
Hamilton Consulting Engineers, Inc.

Prepared For:
City of Washington, Illinois

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3230 Executive Drive
Joliet, IL 60431-8401

phone: 815-730-3444
fax: 815-730-6703

MEMORANDUM

DATE: June 22, 2023
TO: City of Washington, IL
FROM: Howard Hamilton, PE, CFM, CPESC
SUBJECT: Alternative Analysis, Draft Concerns/Updates and
Smoke Test Discussion
HCE # 21911

ALTERNATIVE ANALYSIS

In response to the City's questions regarding Alternatives Analysis prepared by Hamilton Consulting Engineers, Inc. (HCE), we are pleased to provide additional context and answers to the Council. It is our understanding that the Council met to discuss the report on Monday, March 20, 2023, and expressed the following concerns:

1. Apparent errors and/or discrepancies with the Preliminary Engineer's Opinion of Probable Construction Costs (PEOPCC) for both the L-1 and E-3 alignments.
2. Revisions to Alignment E-3 from the original location proposed by Aptim/Goat Springs, LLC.

Background

HCE completed the first, *Draft* version, of the analysis, titled the "Farm Creek Trunk Sewer 3rd Party Alignment Analysis" on February 15, 2022, and presented our finding to the City Council on February 21, 2022. As outlined in our proposal and through discussions with City staff, HCE would present a draft version of the analysis to the city, finalize the report over the next one to two weeks, and then release a final version for public review and comment. While the substance of the final version would not change in a material way from the draft version, the estimates, exhibits, and text would continue to be refined, and input from the Council and City staff would be incorporated. At the direction of the city, HCE ceased all work after presenting to Draft version to the Council and HCE made no further edits.

Estimates

Specifically, the factor resulting in apparent errors is the draft nature of both the estimate and plans, specifically regarding the method of construction for each length of sewer.

Engineering is an iterative process involving trial runs, estimates and analyses, and then revisions. Through this process, HCE laid out the L-1 and E-3 alignments, provided manholes at key locations, and identified obvious locations where construction would use directional boring (at extreme sewer depths) or jack and bore methods (at sensitive crossings like creeks and railroads). This information was conveyed on the Plan and Profile drawings for each alignment.

Environmental Design International inc.

has combined with

Hamilton Consulting Engineers, Inc.

Effective June 1, 2023

Howard J. Hamilton PE, CFM, CPESC

Director of Civil Engineering



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ALTERNATIVE ANALYSIS

In response to the City's questions regarding Alternative Engineers, Inc. (HCE), we are pleased to provide additional understanding that that the Council met to discuss and expressed the following concerns:

1. Apparent errors and/or discrepancies with the Construction Costs (PEOPCC) for both the L-1 and E-3
2. Revisions to Alignment E-3 from the original layout

Background

HCE completed the first, *Draft* version, of the analysis "Alignment Analysis" on February 15, 2022, and presented it on February 21, 2022. As outlined in our proposal and through discussion, the draft version of the analysis to the city, finalize the report, release a final version for public review and comment, and not change in a material way from the draft version, to be refined, and input from the Council and City staff. After the city, HCE ceased all work after presenting to Draft version and edits.

Estimates

Specifically, the factor resulting in apparent errors is related to the method of construction for the L-1 and E-3.

Engineering is an iterative process involving trial runs and adjustments. Through this process, HCE laid out the L-1 and E-3 alignments and identified obvious locations where construction would be required (at depths) or jack and bore methods (at sensitive crossings). This was conveyed on the Plan and Profile drawings for the L-1 and E-3.

Purpose of Tonight's Discussion

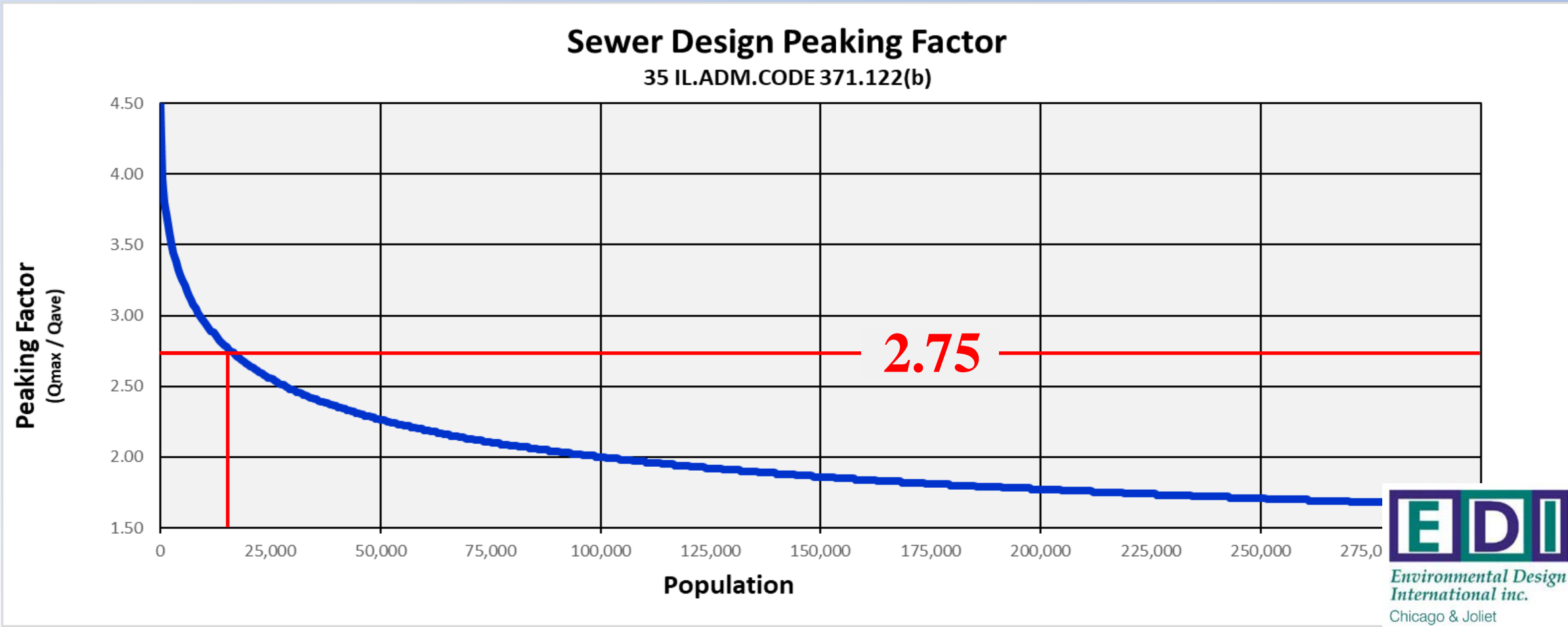
1. Provide "brief" review of work and findings to date
2. Discuss questions that are addressed in the Memo
3. Question and Answer

Some Definitions

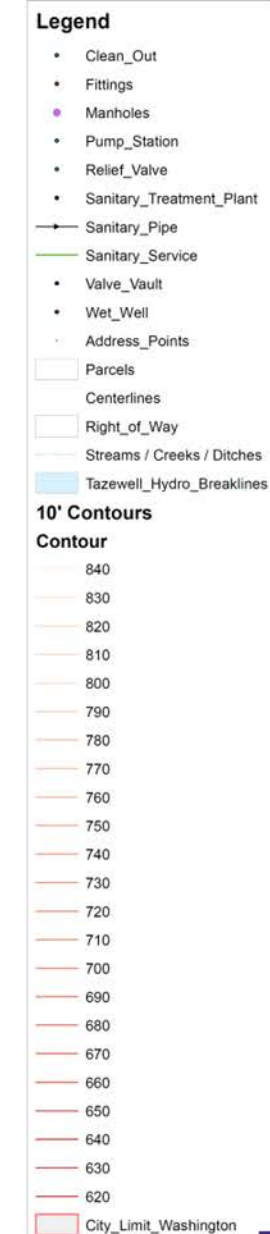
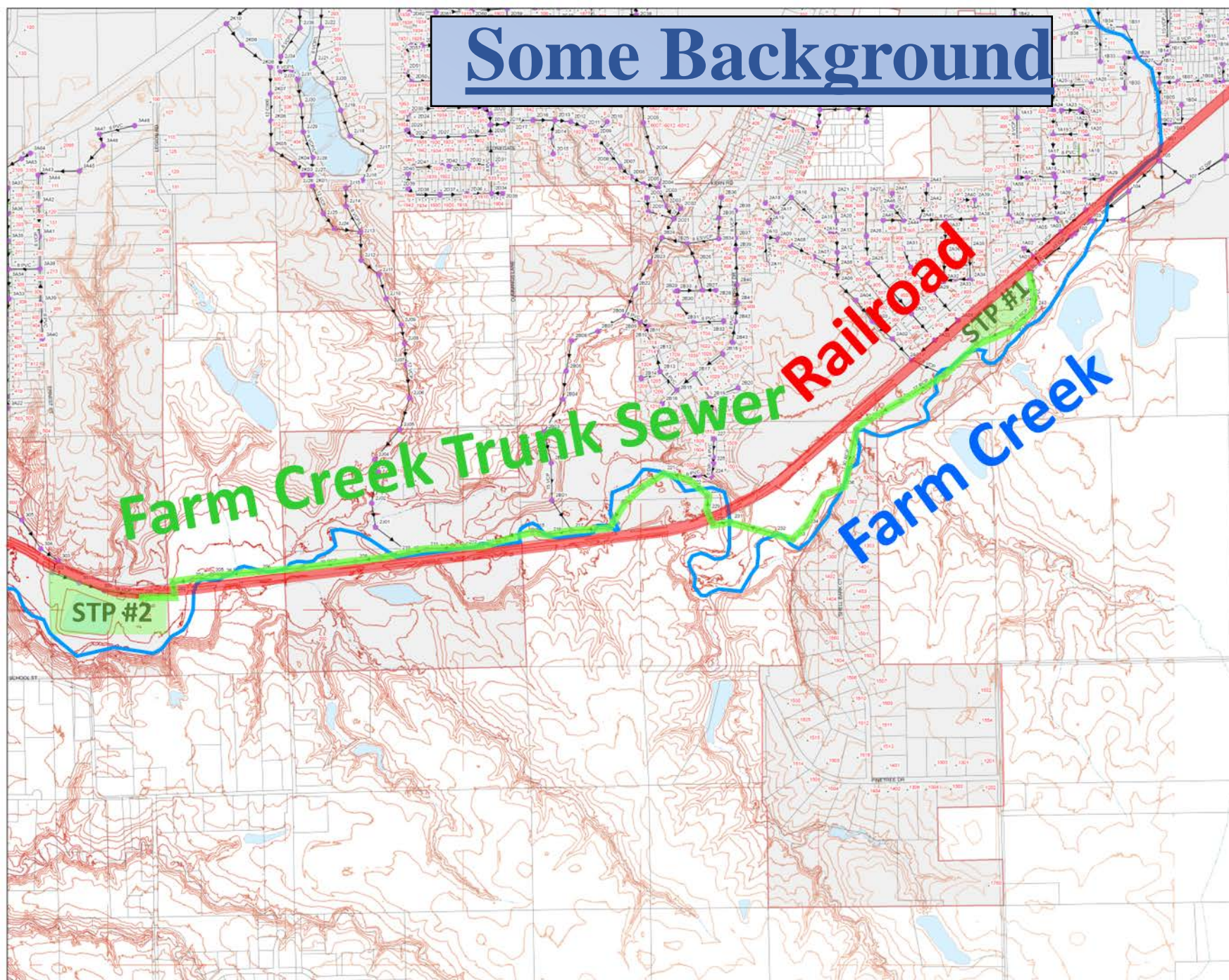
GPM	= Gallons Per Minute
GPD	= Gallons Per Day
MGD	= Million Gallons Per Day
I/I	= Infiltration and Inflow
Infiltration	= The groundwater that seeps into leaky sewers
Inflow	= The stormwater that flows into open sewers
PE	= Population Equivalent
	100 gallons of sewage per day per PE
	70 – 80 gallons sewage
	20 – 30 gallons I/I

Some Definitions

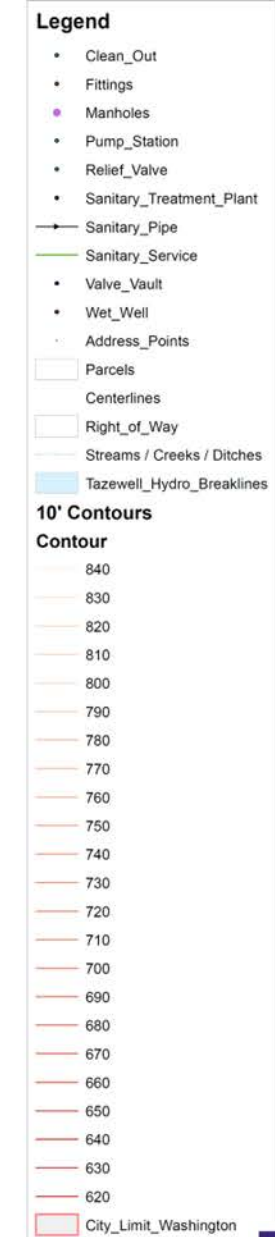
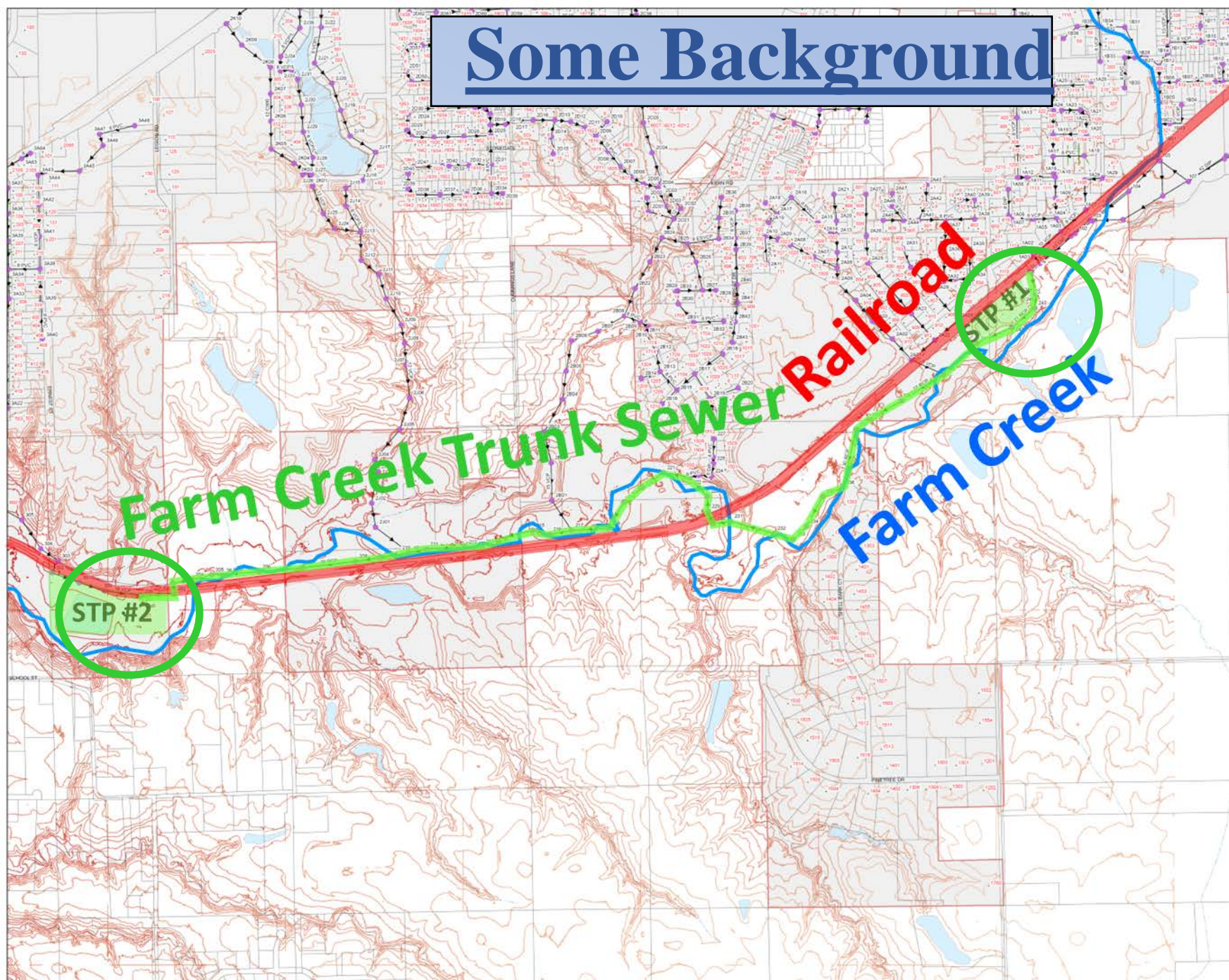
Peaking Factor = The ratio of the peak flow over the average flow in a sewer



Some Background



Some Background



Some Background

- 1950** STP #1 Constructed
- 1971** STP #2 and Farm Creek Trunk Sewer (FCTS)
- 2015** FCTS Easements reviewed
- 2016** Strand retained for FCTS Replacement Project
- 2019** Strand conducts a systemwide flow study to size FCTS
- 2020** Property Owners adjacent to FCTS express concerns
- 2021** Strand and Property Owner Representatives prepare competing alignment alternatives for the Farm Creek Trunk Sewer

11/4/2021

HCE Retained to Evaluate FCTS Alternatives:

1. Collect Existing Data
2. Interview City Staff
3. Interview Property Owners
4. Community Survey and Website
5. Existing FCTS Evaluation from Existing Data
6. Draft Report
7. Report Revisions
8. Public Hearing
9. Presentation to Council
10. Final Draft
11. Final Report
12. Contingency, Allowance

11/4/2021 HCE Retained to Evaluate FCTS Alternatives:

1. Collect Existing Data

12/14/2021  2. Interview City Staff
3. Interview Property Owners

1/18 – 2/28 4. Community Survey and Website
5. Existing FCTS Evaluation from Existing Data

6. Draft Report

~~7. Report Revisions~~

~~8. Public Hearing~~

2/21/2022  9. Presentation to Council

~~10. Final Draft~~

~~11. Final Report~~

12. Contingency, Allowance

**Memo Re: Alignment
and Estimates,
Smoke Test
Discussion**

Key Points

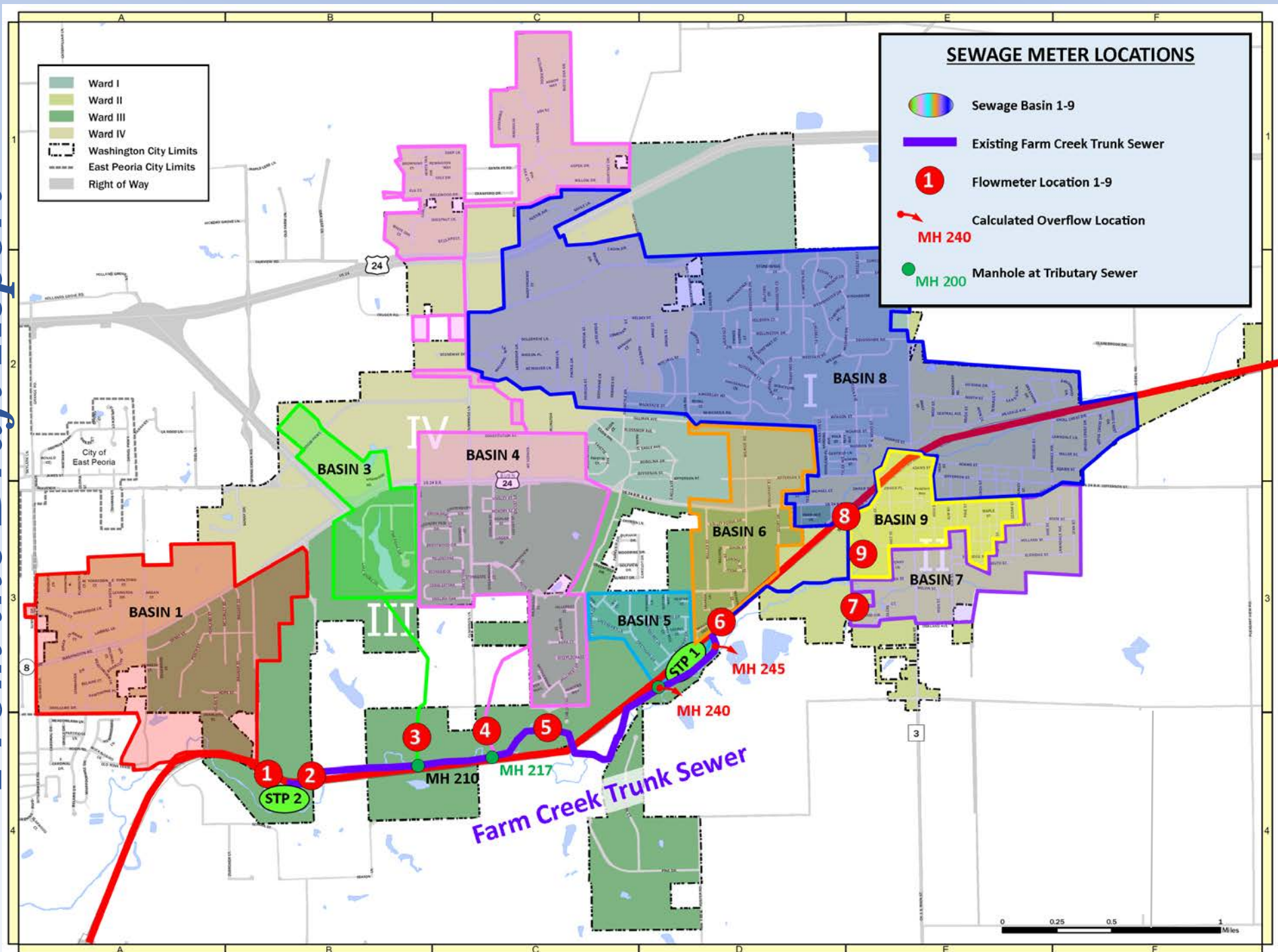
From the Draft Report

Sewage Flows

1. Wet Weather flows from the City sewerage basins into FCTS are excessive
2. Flows recorded August 30, 2016 were equivalent to peak flow expected from a town of 92,230 people (PE)
3. Excess flows are generated from older areas in town, NOT due to the condition of FCTS

Key Points

From the Draft Report



Sewerage Basins

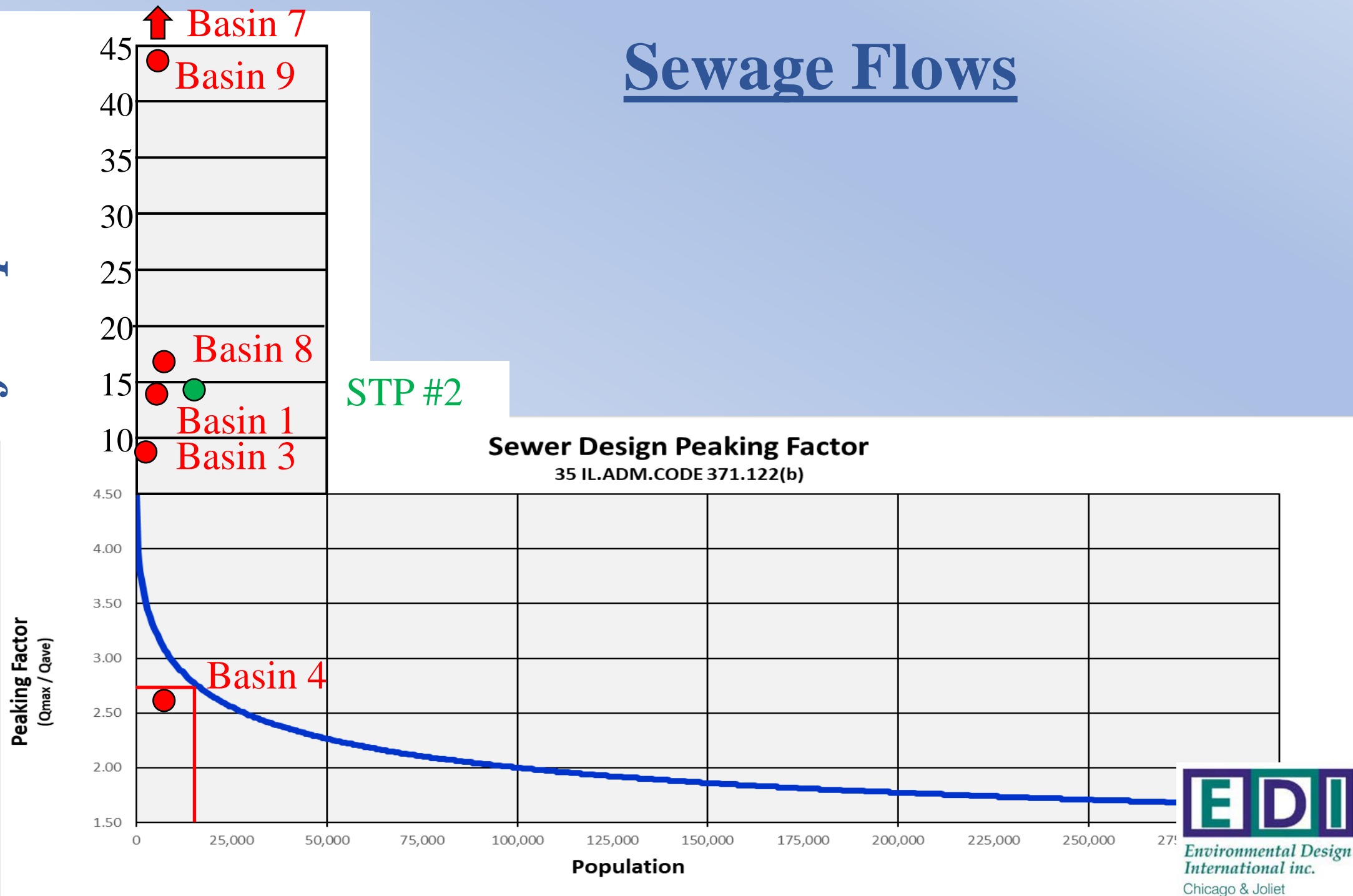


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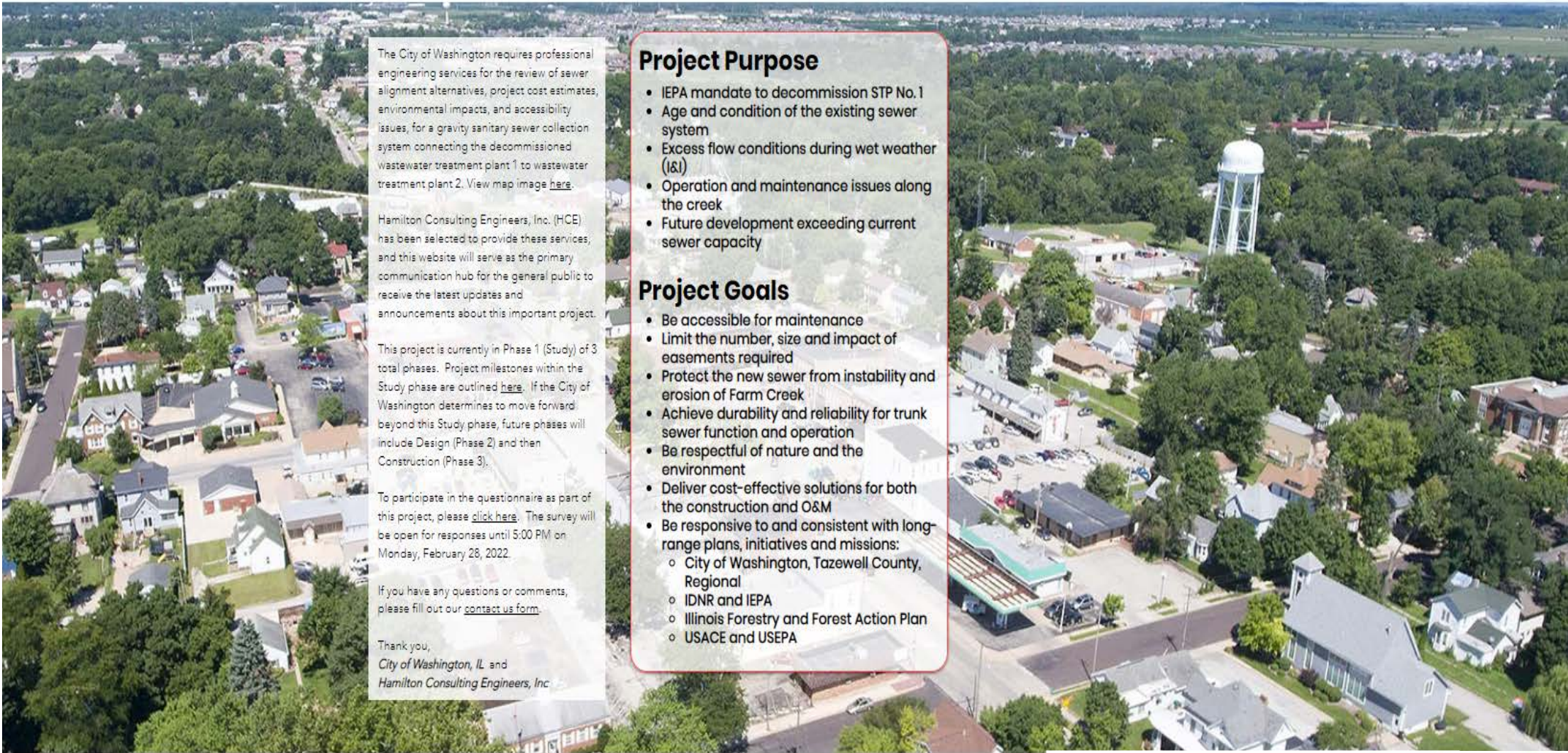
Key Points

From the Draft Report

Sewage Flows



Website



The City of Washington requires professional engineering services for the review of sewer alignment alternatives, project cost estimates, environmental impacts, and accessibility issues, for a gravity sanitary sewer collection system connecting the decommissioned wastewater treatment plant 1 to wastewater treatment plant 2. View map image [here](#).

Hamilton Consulting Engineers, Inc. (HCE) has been selected to provide these services, and this website will serve as the primary communication hub for the general public to receive the latest updates and announcements about this important project.

This project is currently in Phase 1 (Study) of 3 total phases. Project milestones within the Study phase are outlined [here](#). If the City of Washington determines to move forward beyond this Study phase, future phases will include Design (Phase 2) and then Construction (Phase 3).

To participate in the questionnaire as part of this project, please [click here](#). The survey will be open for responses until 5:00 PM on Monday, February 28, 2022.

If you have any questions or comments, please fill out our [contact us form](#).

Thank you,
City of Washington, IL and
Hamilton Consulting Engineers, Inc.

Project Purpose

- IEPA mandate to decommission STP No. 1
- Age and condition of the existing sewer system
- Excess flow conditions during wet weather (I&I)
- Operation and maintenance issues along the creek
- Future development exceeding current sewer capacity

Project Goals

- Be accessible for maintenance
- Limit the number, size and impact of easements required
- Protect the new sewer from instability and erosion of Farm Creek
- Achieve durability and reliability for trunk sewer function and operation
- Be respectful of nature and the environment
- Deliver cost-effective solutions for both the construction and O&M
- Be responsive to and consistent with long-range plans, initiatives and missions:
 - City of Washington, Tazewell County, Regional
 - IDNR and IEPA
 - Illinois Forestry and Forest Action Plan
 - USACE and USEPA

The City of Washington requires professional engineering services for the review of sewer alignment alternatives, project cost estimates, environmental impacts, and accessibility issues, for a gravity sanitary sewer collection system connecting the decommissioned wastewater treatment plant 1 to wastewater

Project Purpose

- IEPA mandate to decommission STP No. 1
- Age and condition of the existing sewer system
- Excess flow conditions during wet weather (sf)

Purpose of the Website

1. Provide project overview to the public
2. Provide open library of project-related documents
3. Provide link for questions and comments
4. Collect information and opinions via online questionnaire*

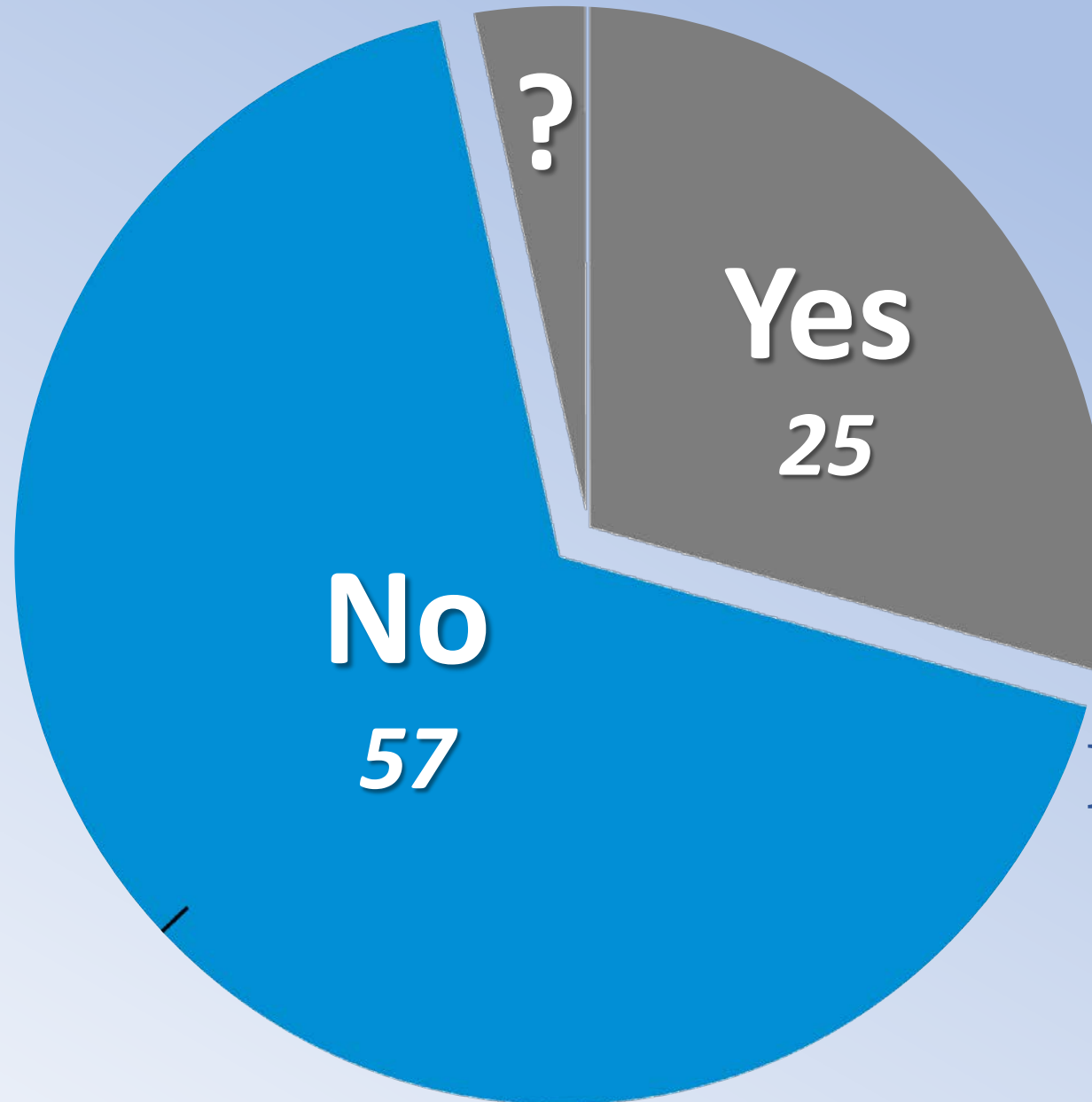
**Questionnaire was available from January 18 - February 28, 2022*

Responses provided here collected through February 12, 2022

Website

Key Points

From the Draft Report



*Have you
experienced
sewer
backups?*

**Out of 150
Respondents**

Key Points

From the Draft Report

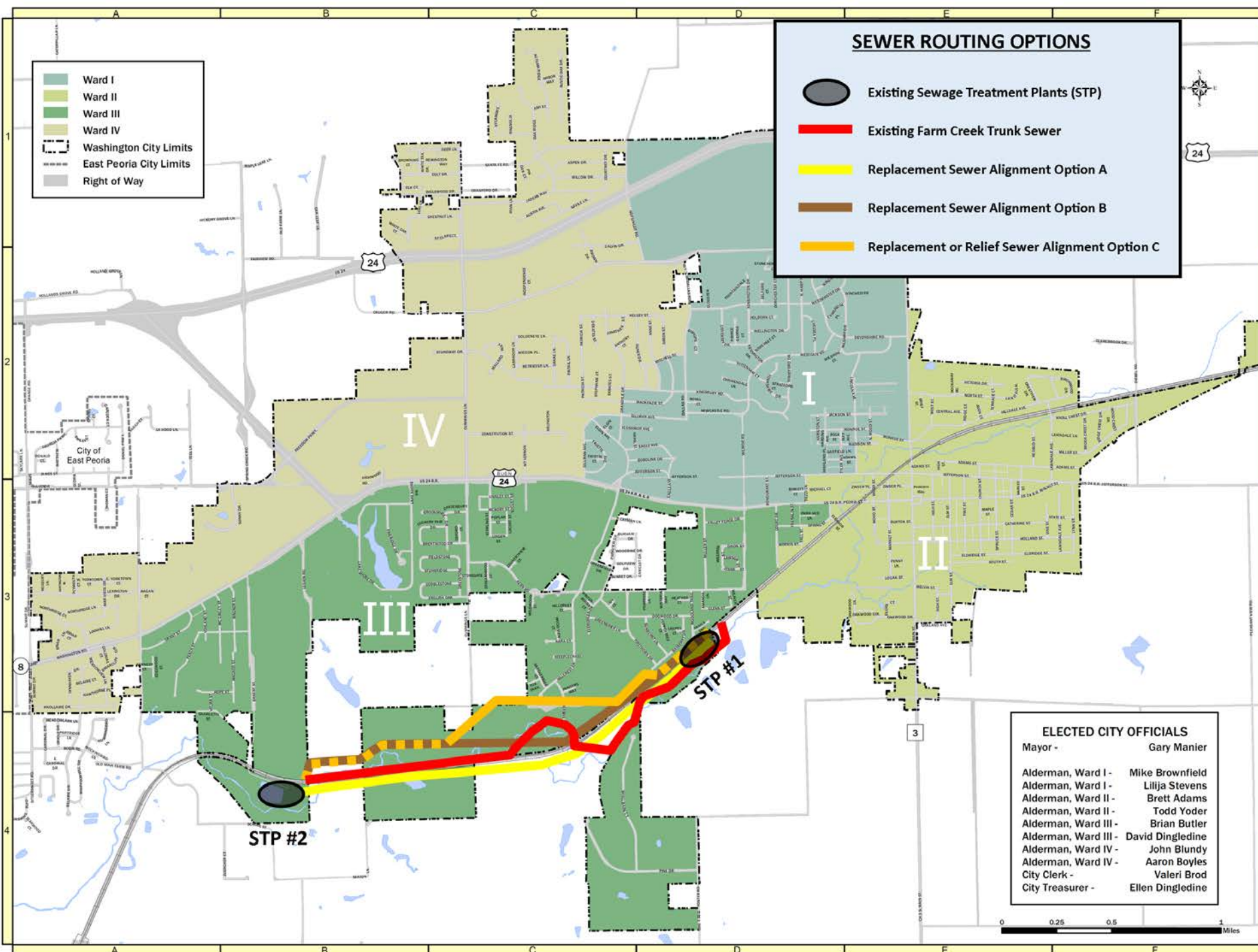
Seven Farm Creek Trunk
Sewer Alternatives were
Evaluated,
Designated A-G

*Pump station improvements at STP#2 are required
at \$3,000,000^{CE} regardless of the chosen alternative*

Alternatives From the Draft Report



Chicago & Joliet



CITY OF WASHINGTON

TAZEWELL COUNTY, ILLINOIS

City Wards

STREET INDEX

A	ADAMS ST. E2 - F2	H	HALE ST. A1	P	PAERL ST. E2
B	ADAMS ST. W1 - D1	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
C	ADAMS ST. E2 - F2	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
D	ADAMS ST. W1 - D1	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
E	ADAMS ST. E2 - F2	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
F	ADAMS ST. W1 - D1	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
G	ADAMS ST. E2 - F2	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
H	ADAMS ST. W1 - D1	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
I	ADAMS ST. E2 - F2	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
J	ADAMS ST. W1 - D1	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
K	ADAMS ST. E2 - F2	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
L	ADAMS ST. W1 - D1	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
M	ADAMS ST. E2 - F2	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
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Q	ADAMS ST. E2 - F2	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
R	ADAMS ST. W1 - D1	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
S	ADAMS ST. E2 - F2	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
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U	ADAMS ST. E2 - F2	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
V	ADAMS ST. W1 - D1	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
W	ADAMS ST. E2 - F2	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
X	ADAMS ST. W1 - D1	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
Y	ADAMS ST. E2 - F2	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1
Z	ADAMS ST. W1 - D1	H	HAMILTON DR. D2	P	PAERL ST. W1 - D1

ELECTED CITY OFFICIALS

Mayor -	Gary Manier
Alderman, Ward I -	Mike Brownfield
Alderman, Ward I -	Lilija Stevens
Alderman, Ward II -	Brett Adams
Alderman, Ward II -	Todd Yoder
Alderman, Ward III -	Brian Butler
Alderman, Ward III -	David Dingledine
Alderman, Ward IV -	John Blundy
Alderman, Ward IV -	Aaron Boyles
City Clerk -	Valeri Brod
City Treasurer -	Ellen Dingledine

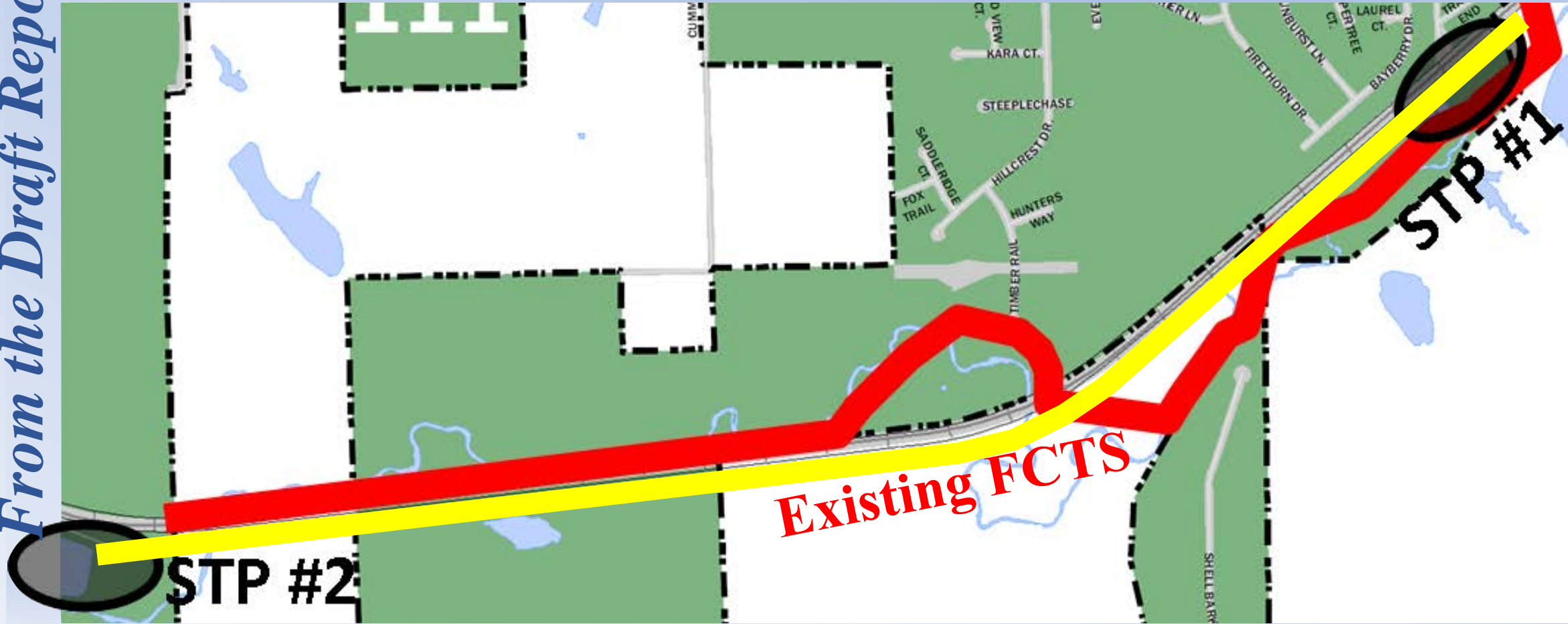
Prepared by the City of Washington Department of Planning and Development: 9/11/2020.

This map indicates approximate street right-of-way and may not be 100% accurate. Newly constructed streets or recently annexed areas may not be shown.

Alternatives

From the Draft Report

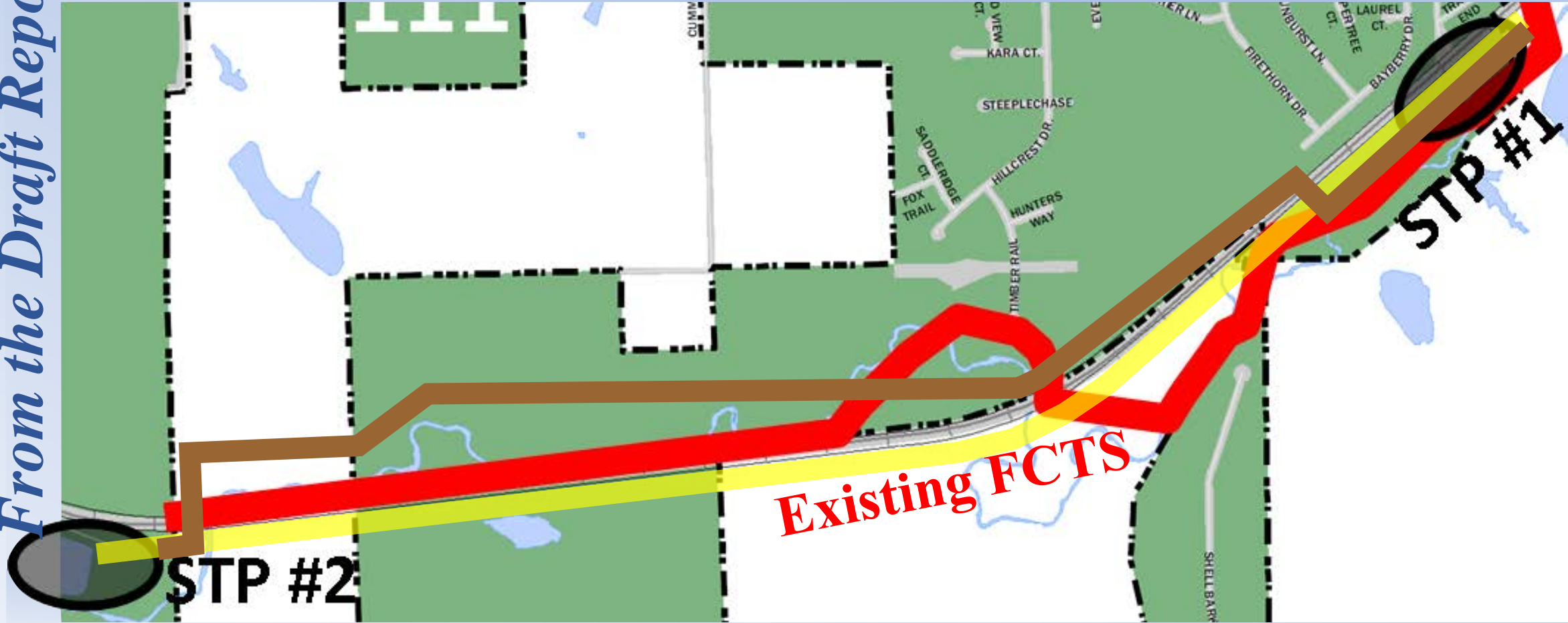
ALTERNATIVE A Strand Alignment B



ALTERNATIVE A **Strand Alignment B**

- 42” Sewer along south side of Railroad
- Abandon existing FCTS
- Capacity of 21,437 gpm, 169,611 PE
- 90% Designed
- Easements required
- Does not reduce excess flows
- \$8,000,000^{CE} plus \$3,000,000^{CE} pump station at STP#2 and FCTS abandonment costs

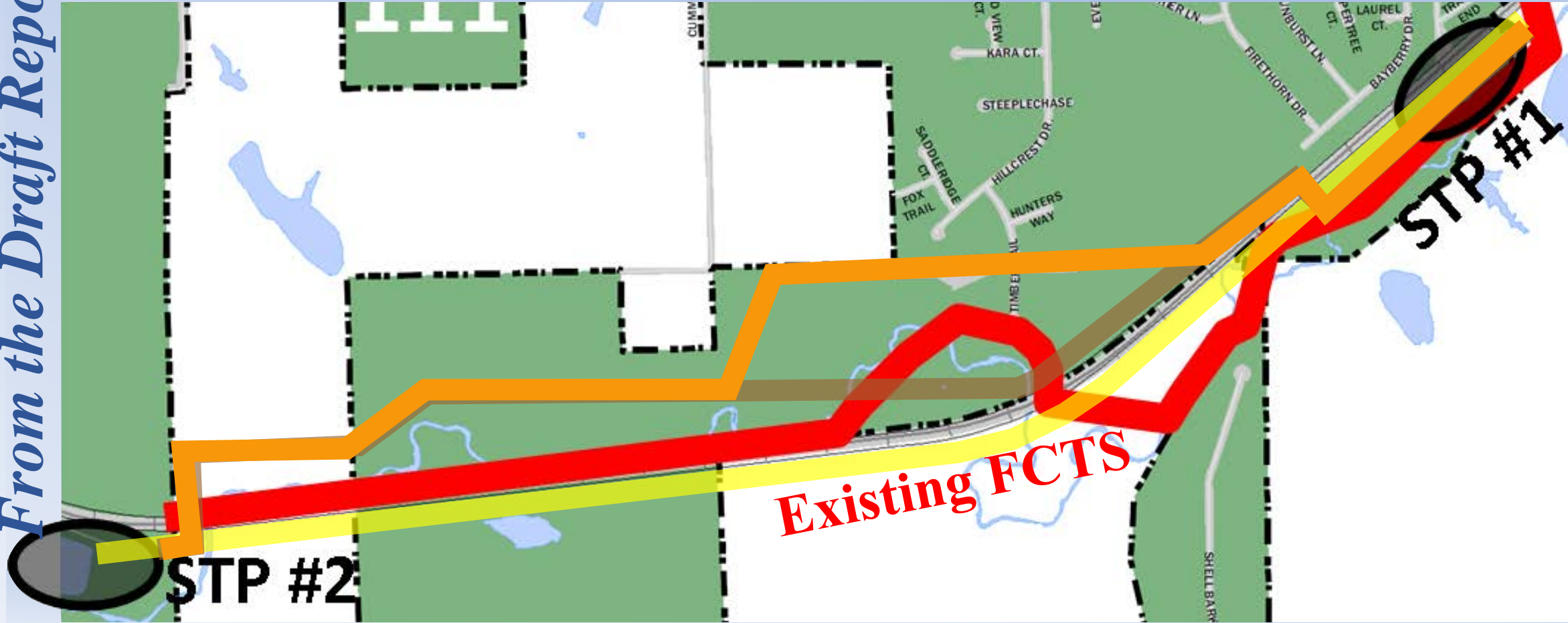
ALTERNATIVE B Pudik Alignment L-1



ALTERNATIVE B **Pudik Alignment L-1**

- 42” Sewer north of Farm Creek
- Abandon existing FCTS
- Capacity of 21,437 gpm, 169,611 PE
- Easements required
- Does not reduce excess flows
- Does not serve areas south of the railroad
- \$10,980,000^{CE}
plus \$3,000,000^{CE} pump station at STP#2 and
FCTS abandonment costs

ALTERNATIVE C Pudik Alignment E-3

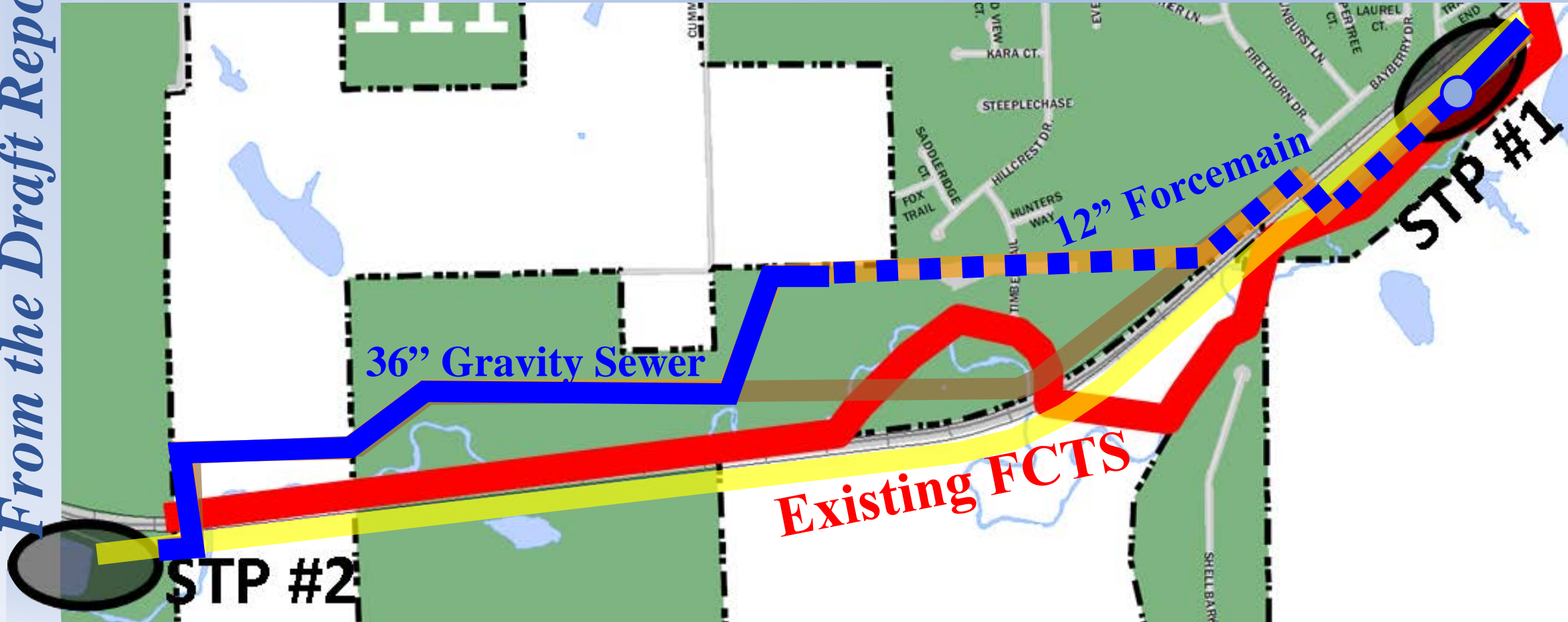


ALTERNATIVE C **Pudik Alignment E-3**

- 42” Sewer north of Farm Creek
- Abandon existing FCTS
- Capacity of 21,437 gpm, 169,611 PE
- Easements required
- Does not reduce excess flows
- Does not serve areas south of the railroad
- Excessive depth
- \$12,581,197^{CE}
plus \$3,000,000^{CE} pump station at STP#2 and
FCTS abandonment costs

ALTERNATIVE D

Pump Station, Forcemain & Relief Sewer

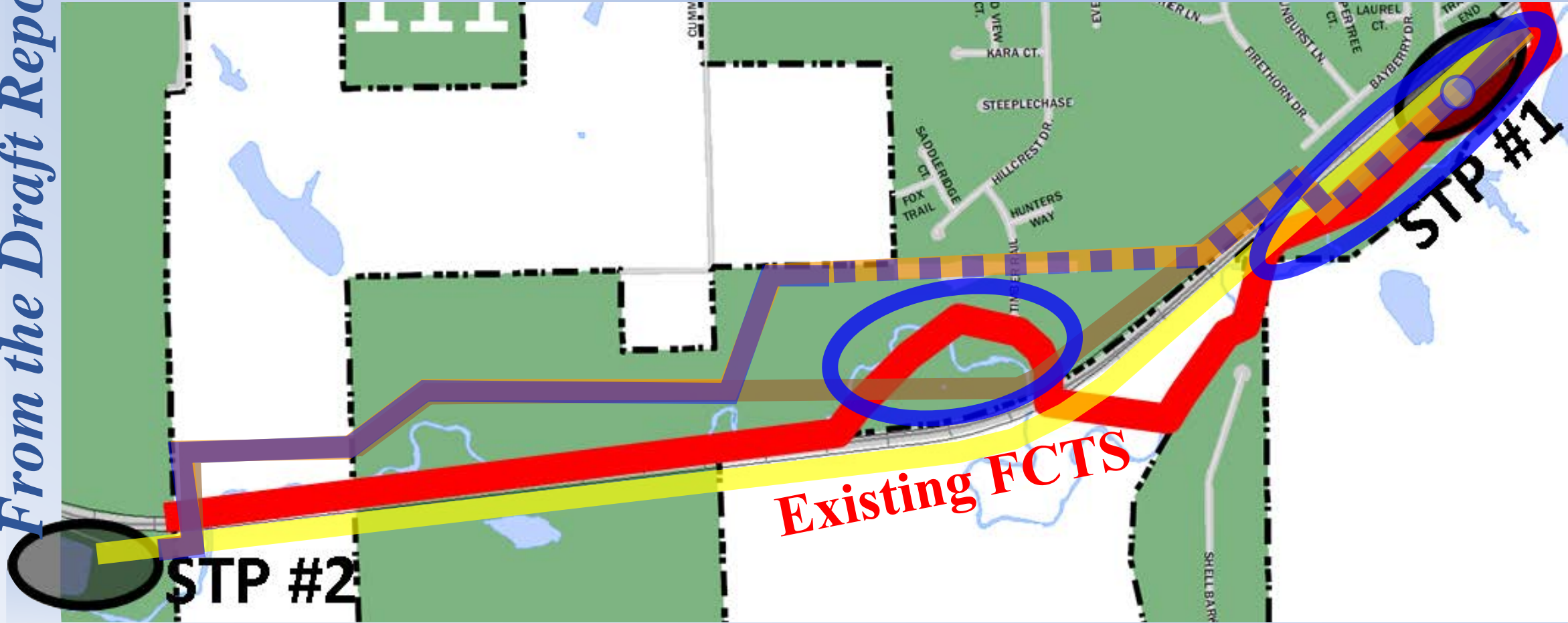


ALTERNATIVE D

Pump Station, Forcemain & Relief Sewer

- Pump Station to offload high flows from FCTS
- Keep existing FCTS with evaluation, repair as needed
- Capacity of 12,973 gpm + FCTS at 4,645 gpm = 17,618 gpm, equivalent to 129,434 PE
- Easements required
- Does not reduce excess flows, but pump station can be downsized as excess flows are reduced systemwide
- Improved service north and south
- \$7,618,040^{CE} plus \$3,000,000^{CE} pump station at STP#2 and FCTS evaluation and repair

ALTERNATIVE E Relief Sewers



ALTERNATIVE E **Relief Sewers**

1. Sewer appears to be in generally good condition, although internal inspection and repair as needed is recommended
2. Sewer has two “bottlenecks”:
 - First bottleneck has manholes overflowing during precipitation events
 - Second bottleneck severely limits capacity of sewer

ALTERNATIVE E **Relief Sewers**

Existing Farm Creek Trunk Sewer Bottleneck #1

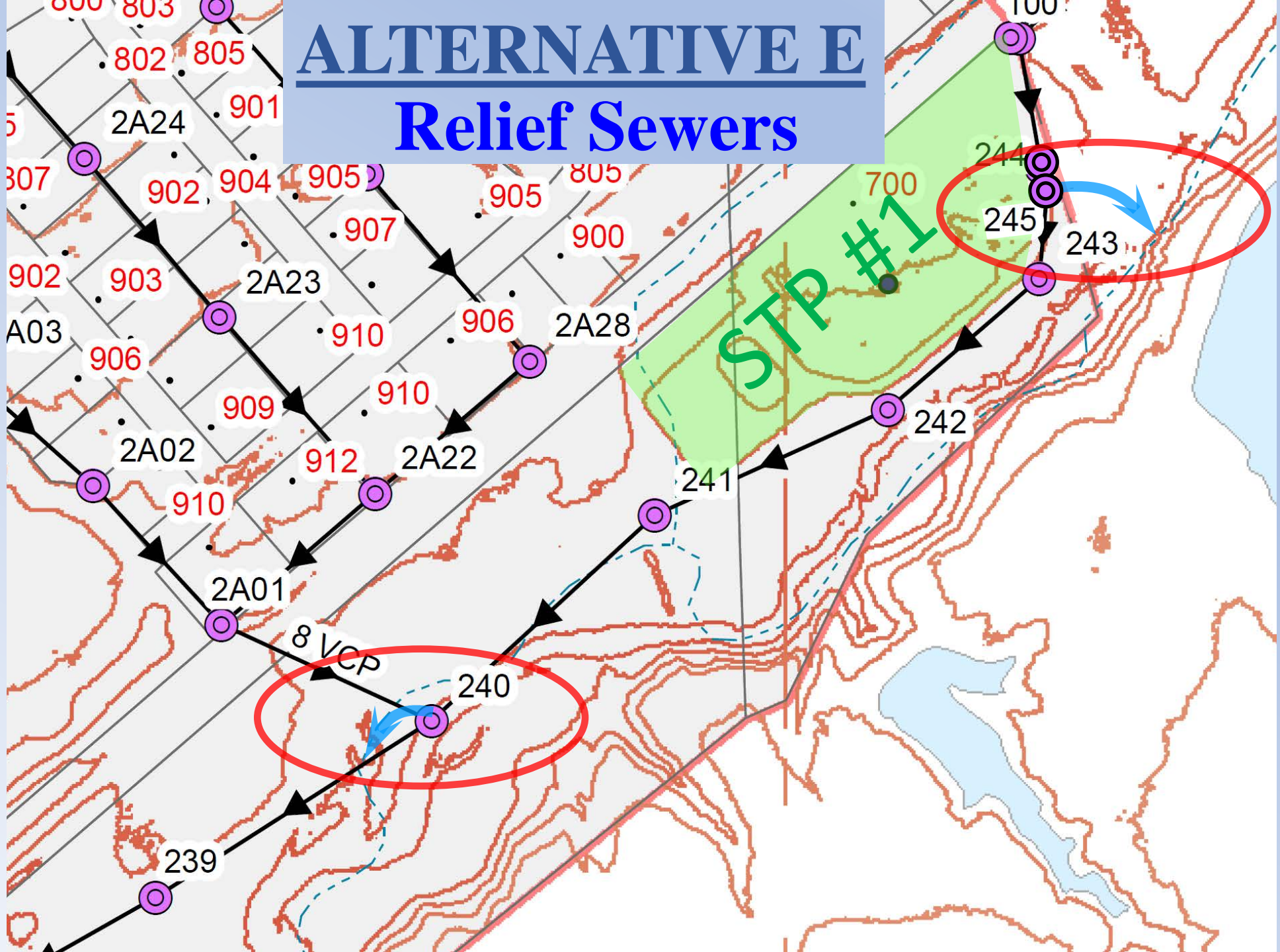
3. The length of sewer between Manholes 238 and 245 has a capacity of only 5,882 gallons per minute (gpm) which is enough for peak flow from 35,145 PE.

However, the recorded flow on August 30, 2016 was nearly double that amount at 11,671 gpm.

This causes Manholes 245 and 240 to overflow.

Alternatives

From the Draft Report



ALTERNATIVE E **Relief Sewers**

Existing Farm Creek Trunk Sewer Bottleneck #2

4. The length of sewer south of Timber Rail Drive, between Manholes 229 and 219, has capacity of 4,645 gpm, which is enough for the peak flow from 26,443 PE.

However, the recorded flow on August 30, 2016 was nearly 2 ½ times that amount at 11,470 gpm.

Alternatives

From the Draft Report

ALTERNATIVE E Relief Sewers



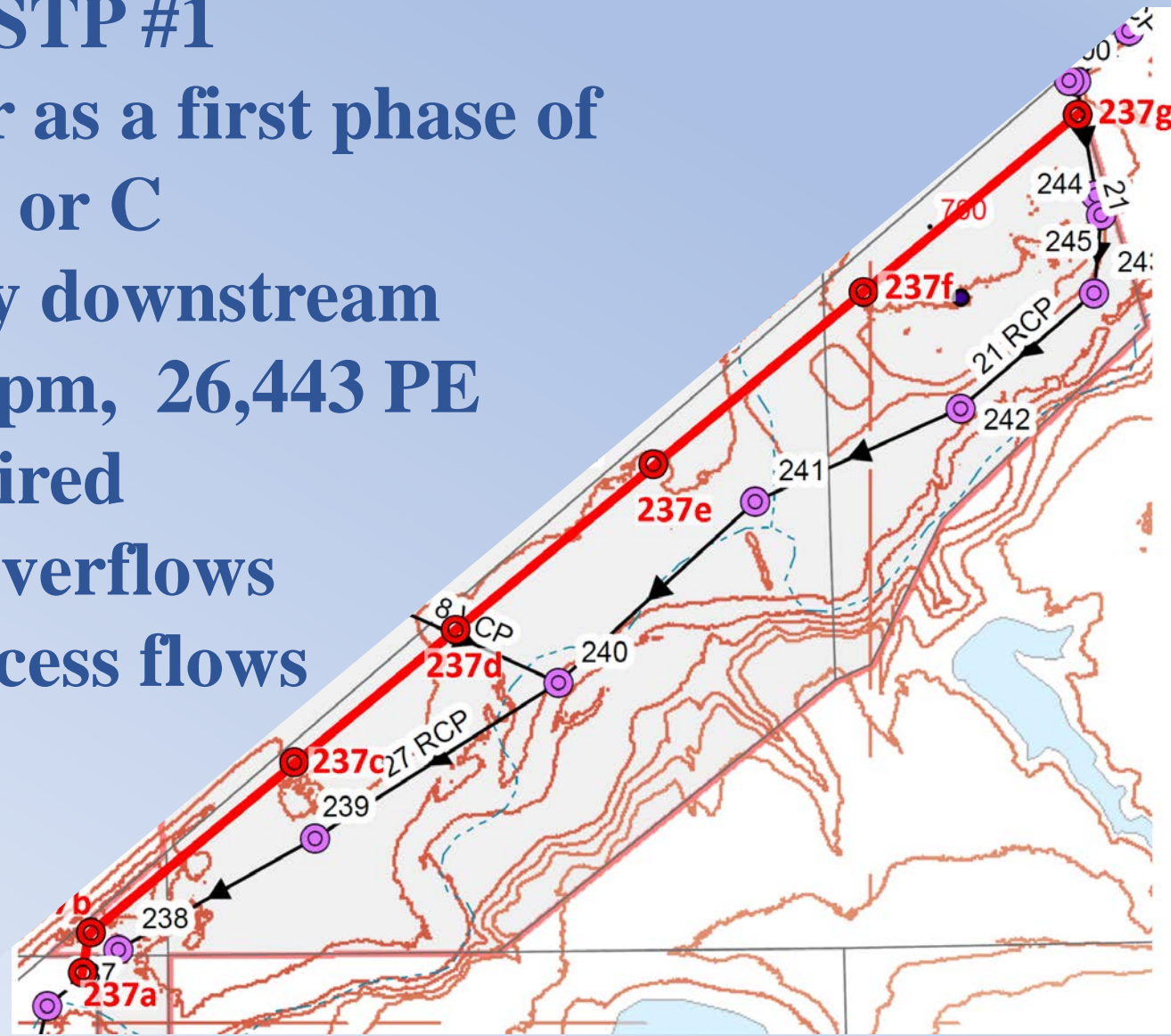
ALTERNATIVE E **Relief Sewers**

5. If the two “bottlenecks” were corrected:
 - Capacity of existing FCTS would expand to 7,826 gpm, enough for the peak flow from 49,648 PE
 - However, this expanded capacity is still 1.5 times less than recorded flow of 11,671 gpm on August 30, 2016 – overflows eliminated
6. Existing FCTS is well-located to provide future service area expansion potential both north and south of Farm Creek and railroad.

ALTERNATIVE E

Relief Sewers, **STP #1 Bypass Sewer**

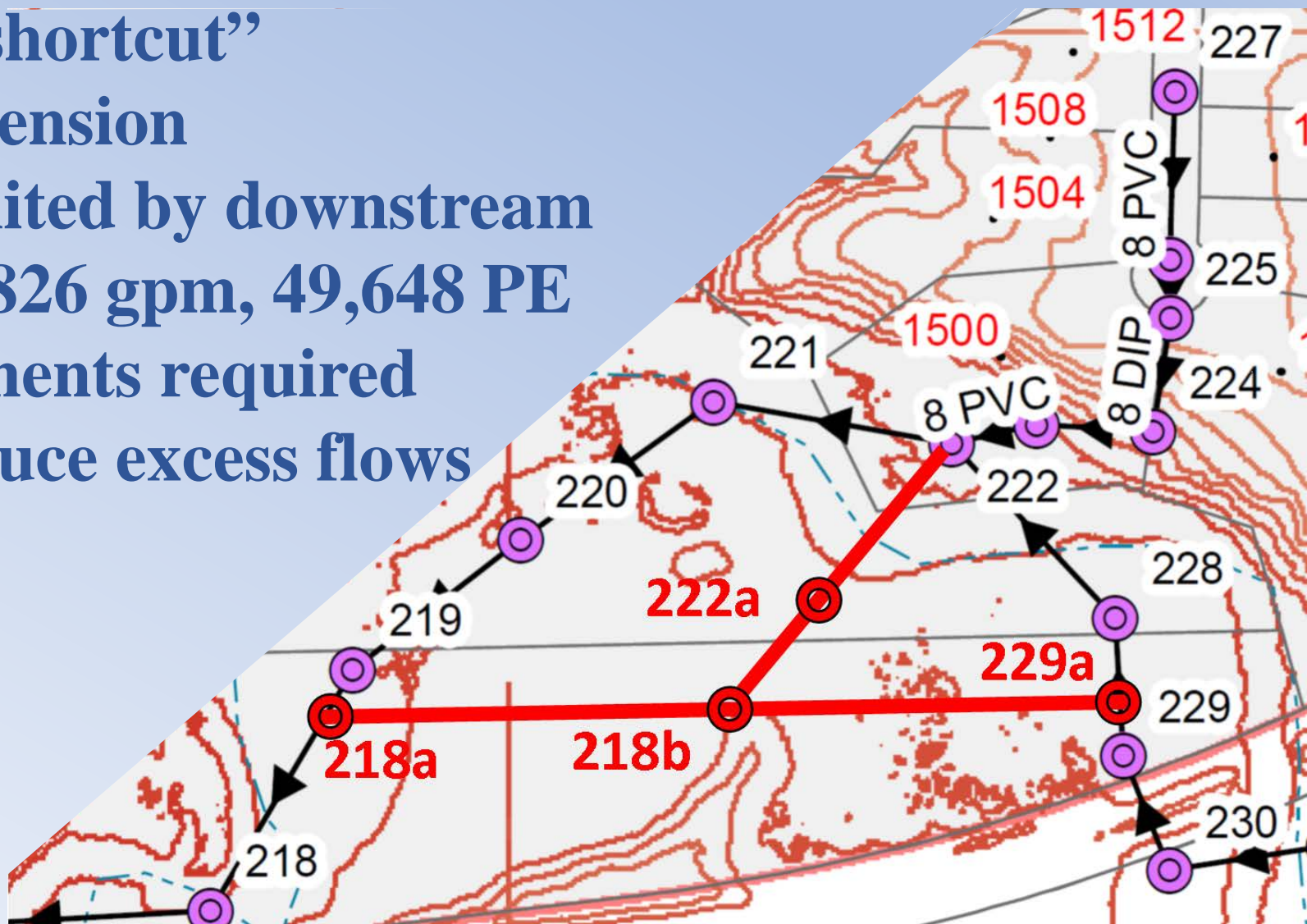
- 30" Sewer around STP #1
- Could be 42" sewer as a first phase of Alternatives A, B, or C
- Capacity limited by downstream sewers to 4,645 gpm, 26,443 PE
- No easements required
- Eliminates FCTS overflows
- Does not reduce excess flows
- \$719,500^{CE}



ALTERNATIVE E

Relief Sewers, Timber Rail Relief Sewer

- 30" Sewer "shortcut"
- 8" Sewer extension
- Capacity limited by downstream sewers to 7,826 gpm, 49,648 PE
- Only 2 easements required
- Does not reduce excess flows
- \$617,712^{CE}



ALTERNATIVE E **Relief Sewers**

- If the entire existing FCTS requires lining, cost could be up to \$3,000,000^{CE}
- \$3,000,000^{CE} pump station at STP#2 is required
- Sanitary Sewer Evaluation Survey (SSES) and I/I removal necessary

ALTERNATIVES F AND G

Alternative F - SSES

- Perform a Sanitary Sewer Evaluation Survey (SSES) and repair all identified sources of I/I
- If successful, this will end the overflows of FCTS, but success is not guaranteed

Alternative G – No Build

- Evaluate and repair FCTS
- Take long-term approach toward eliminating I/I

Neither of these Alternatives are recommended, as neither addresses the immediate issue of FCTS overflows

Recommendations

From the Draft Report

Alternative A

- or -

Alternative E

- Already designed to 90%
- Difficult easements
- SSES advisable
- Sewer oversized for service area (169,611 PE)
- \$8,000,000.00^{CE} *

***Least expensive construction Alternative**

- Small project
- Fewer easements
- SSES required
- Sewer undersized for service area (49,648 PE)
- \$???*

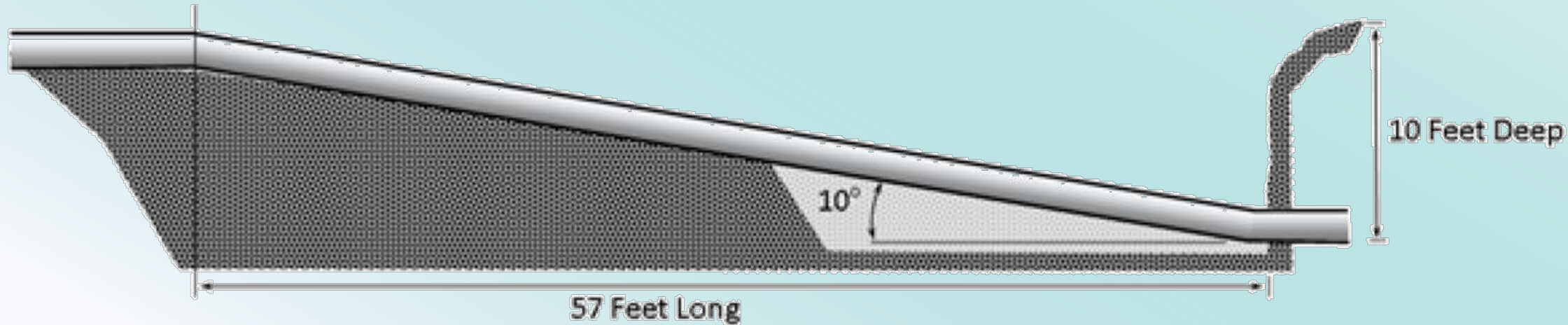
***Potentially the least expensive Alternative**

Memorandum

- 1. Explain apparent errors and/or discrepancies with the Preliminary Engineer's Opinion of Probable Construction Costs (PEOPCC) for both the L-1 and E-3 alignments.**
- 2. Explain revisions to Alignment E-3 from the original location proposed by Aptim/Goat Springs, LLC.**
- 3. Discuss the Smoke Test SSES Report**

1. Preliminary Engineer's Opinion of Probable Construction Costs (PEOPCC)

- Draft report, analysis not 100% complete



- HCE/EDI has since re-evaluated these two alignments

Updated Estimates

	<u>Alternative A</u> <i>Strand</i>	<u>Alternative B</u> <i>L-1</i>	<u>Alternative C</u> <i>E-3</i>
2/15/2022*	\$8,000,000.00 ^{CE}	\$10,980,000.00 ^{CE}	\$12,580,000.00 ^{CE}
6/12/2023**	no change	\$9,570,000.00 ^{CE}	\$11,850,000.00 ^{CE}

^{CE} *Comparative estimate, using EOPCC unit prices from others*

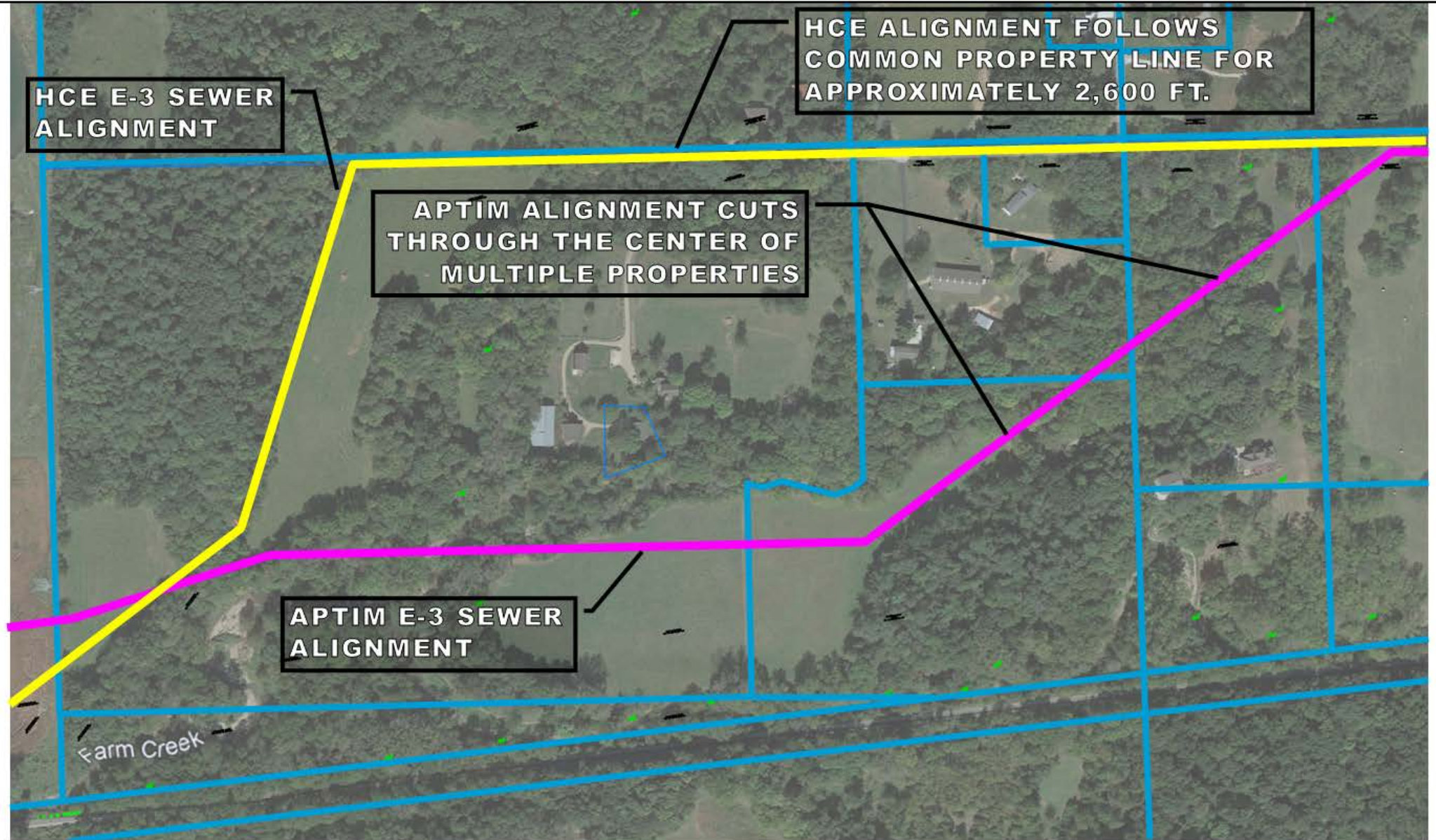
* *Estimates are from the report, not from 2/11/22 presentation to Council which included Pump Station and FCTS Abandonment Estimates*

** *Exclusive of Pump Station and FCTS Abandonment Estimates*

2. Explain revisions to Alignment E-3 from the original location proposed by Aptim/Goat Springs, LLC.

- **Cost**
- **Ease of construction**
- **Acquisition of easements**
- **Impact upon future use of the properties**
- **Ability to extend service to the sewer in the future**

Memorandum



CITY OF WASHINGTON
FARM CREEK TRUNK SEWER 3RD PARTY ALIGNMENT ANALYSIS
ALIGNMENT E-3 COMPARISON

Scale: N.T.S.

Smoke Test Report



City of Washington
2022 Smoke Testing Program
REL Project #22-R0435



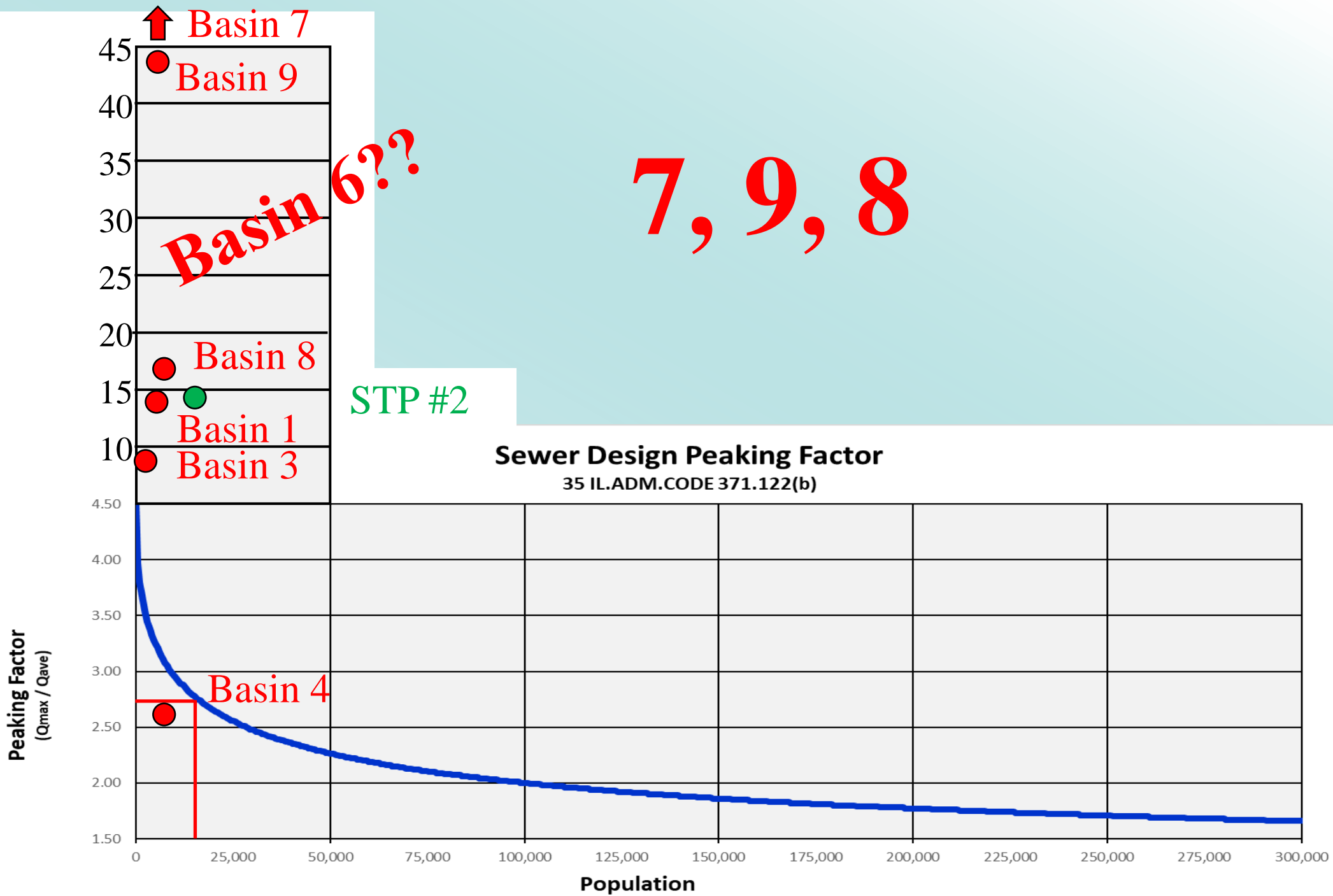
Prepared by:
Robinson Engineering, Ltd.
Joseph Sullivan
630-346-2877
joe.sullivan@reltd.com

June – September 2022

Smoke Test Report



Environmental Design
International inc.
Chicago & Joliet

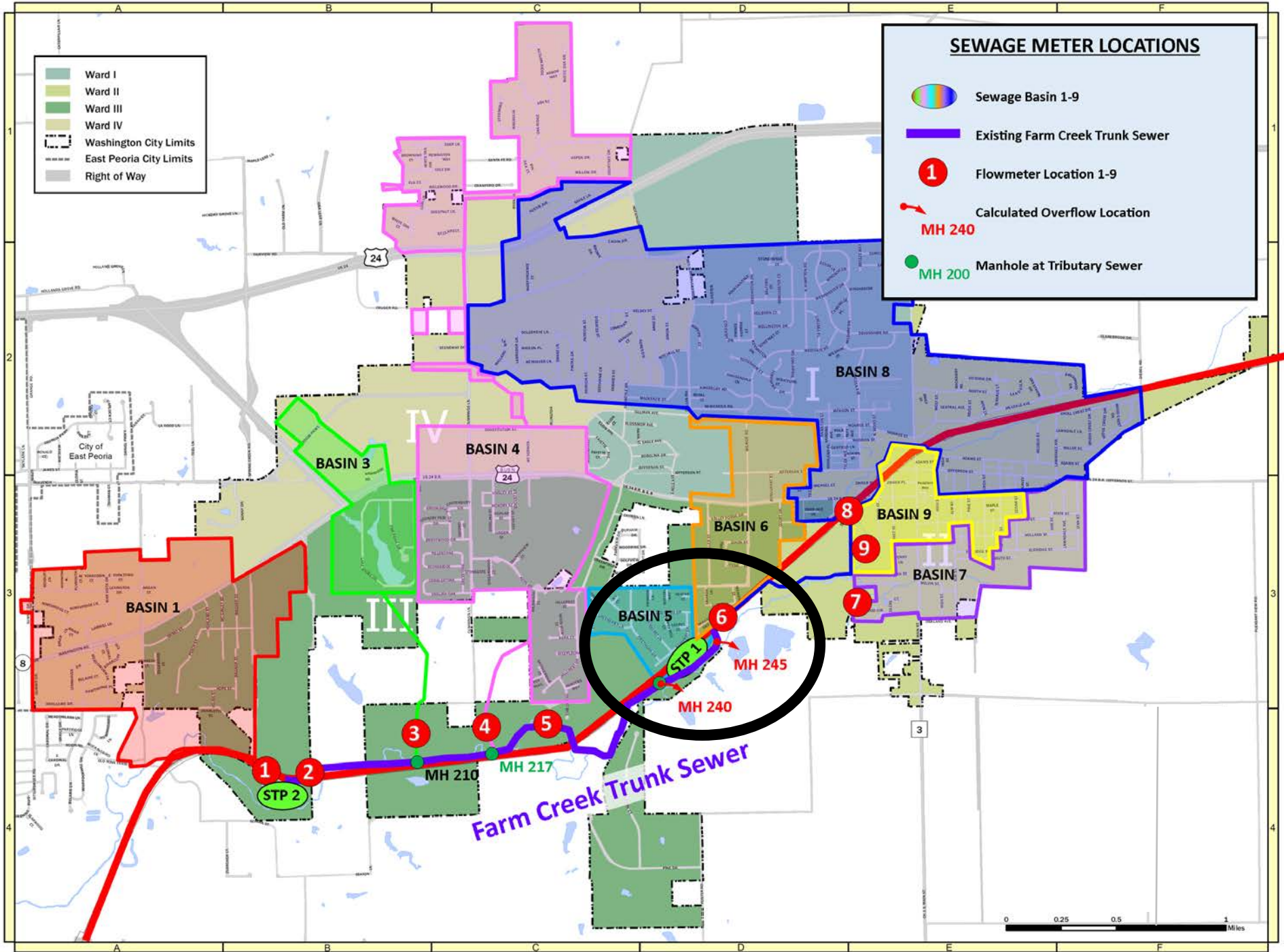


Smoke Test Report



Environmental Design
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Chicago & Joliet



Sewerage Basins

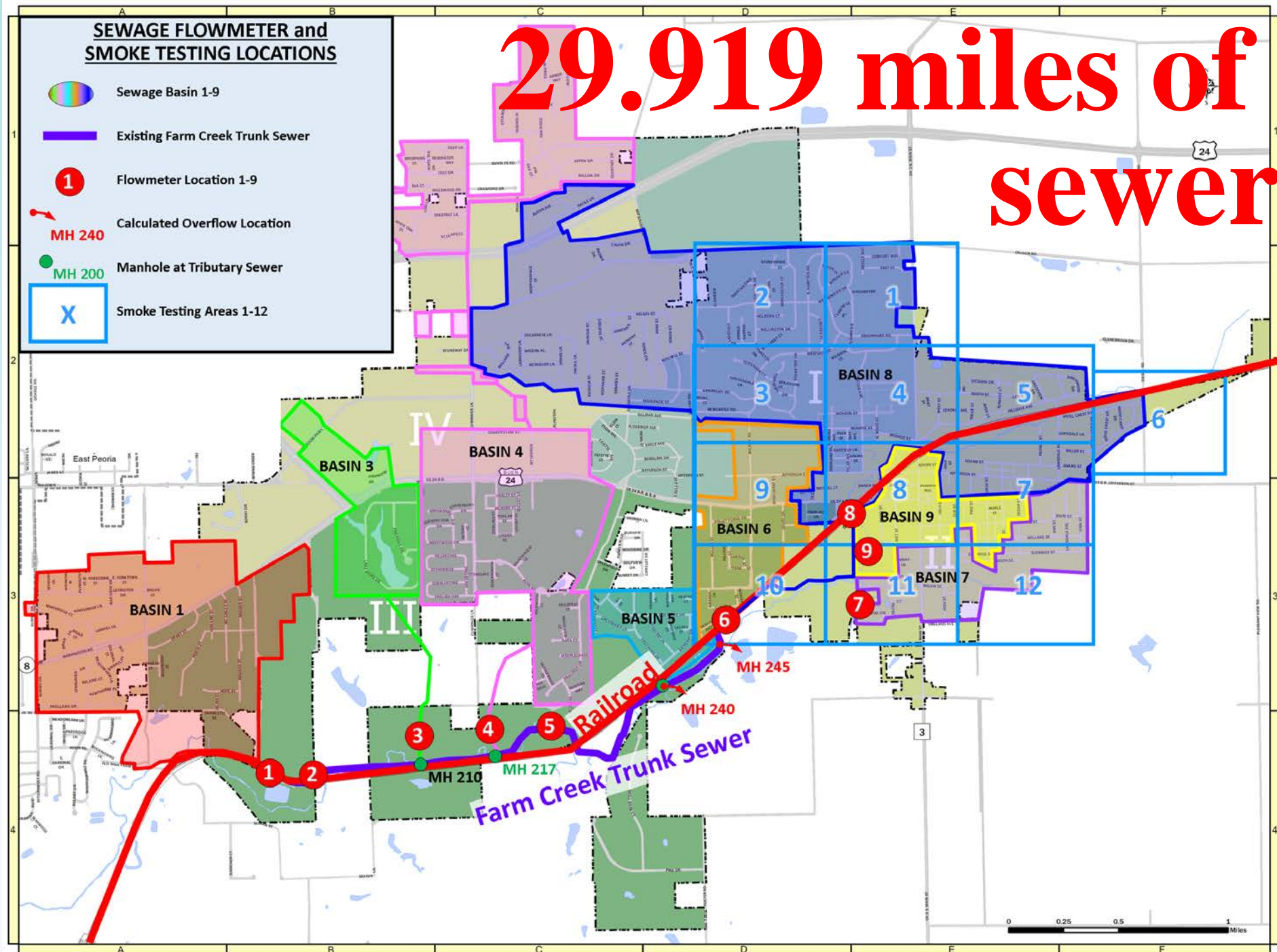


Prepared by the City of Washington Department
of Planning and Development 9/11/2020.

This map indicates approximate street right-of-way
and may not be 100% accurate. Newly constructed
streets or recently annexed areas may not be shown.

Smoke Test Report

29.919 miles of sewer



**CITY OF
WASHINGTON
TAZEWELL COUNTY, ILLINOIS**

City Wards

STREET INDEX

A	ADAMS ST. E2, F2	A	ADAMS ST. E2, F2	A	ADAMS ST. E2, F2
B	ADAMS ST. E2, F2	B	ADAMS ST. E2, F2	B	ADAMS ST. E2, F2
C	ADAMS ST. E2, F2	C	ADAMS ST. E2, F2	C	ADAMS ST. E2, F2
D	ADAMS ST. E2, F2	D	ADAMS ST. E2, F2	D	ADAMS ST. E2, F2
E	ADAMS ST. E2, F2	E	ADAMS ST. E2, F2	E	ADAMS ST. E2, F2
F	ADAMS ST. E2, F2	F	ADAMS ST. E2, F2	F	ADAMS ST. E2, F2
G	ADAMS ST. E2, F2	G	ADAMS ST. E2, F2	G	ADAMS ST. E2, F2
H	ADAMS ST. E2, F2	H	ADAMS ST. E2, F2	H	ADAMS ST. E2, F2
I	ADAMS ST. E2, F2	I	ADAMS ST. E2, F2	I	ADAMS ST. E2, F2
J	ADAMS ST. E2, F2	J	ADAMS ST. E2, F2	J	ADAMS ST. E2, F2
K	ADAMS ST. E2, F2	K	ADAMS ST. E2, F2	K	ADAMS ST. E2, F2
L	ADAMS ST. E2, F2	L	ADAMS ST. E2, F2	L	ADAMS ST. E2, F2
M	ADAMS ST. E2, F2	M	ADAMS ST. E2, F2	M	ADAMS ST. E2, F2
N	ADAMS ST. E2, F2	N	ADAMS ST. E2, F2	N	ADAMS ST. E2, F2
O	ADAMS ST. E2, F2	O	ADAMS ST. E2, F2	O	ADAMS ST. E2, F2
P	ADAMS ST. E2, F2	P	ADAMS ST. E2, F2	P	ADAMS ST. E2, F2
Q	ADAMS ST. E2, F2	Q	ADAMS ST. E2, F2	Q	ADAMS ST. E2, F2
R	ADAMS ST. E2, F2	R	ADAMS ST. E2, F2	R	ADAMS ST. E2, F2
S	ADAMS ST. E2, F2	S	ADAMS ST. E2, F2	S	ADAMS ST. E2, F2
T	ADAMS ST. E2, F2	T	ADAMS ST. E2, F2	T	ADAMS ST. E2, F2
U	ADAMS ST. E2, F2	U	ADAMS ST. E2, F2	U	ADAMS ST. E2, F2
V	ADAMS ST. E2, F2	V	ADAMS ST. E2, F2	V	ADAMS ST. E2, F2
W	ADAMS ST. E2, F2	W	ADAMS ST. E2, F2	W	ADAMS ST. E2, F2
X	ADAMS ST. E2, F2	X	ADAMS ST. E2, F2	X	ADAMS ST. E2, F2
Y	ADAMS ST. E2, F2	Y	ADAMS ST. E2, F2	Y	ADAMS ST. E2, F2
Z	ADAMS ST. E2, F2	Z	ADAMS ST. E2, F2	Z	ADAMS ST. E2, F2



Prepared by the City of Washington Department
of Planning and Development: 9/11/2020.

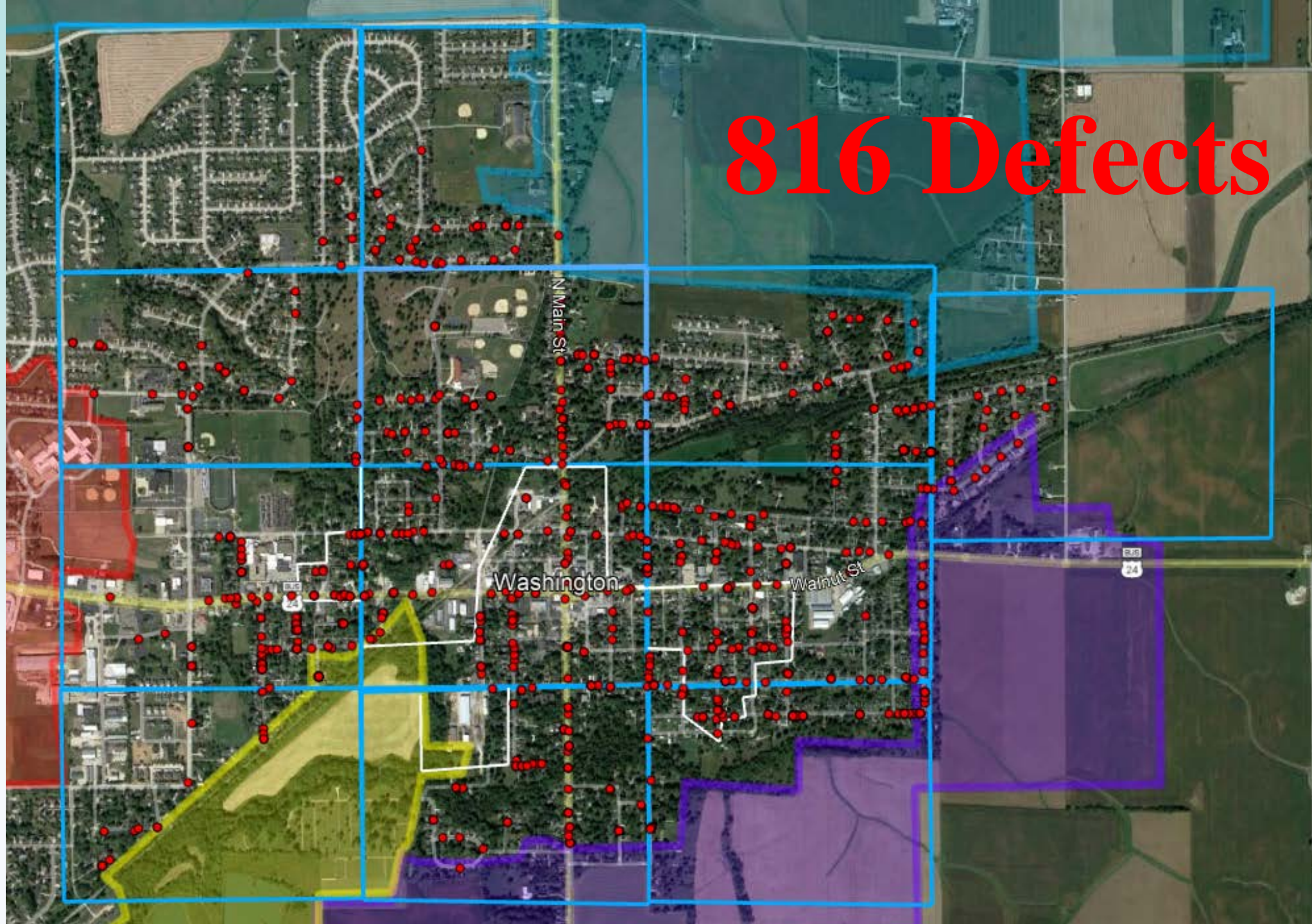
This map indicates approximate street right-of-way
and may not be 100% accurate. Newly constructed
streets or recently annexed areas may not be shown.



Environmental Design
International inc.

Chicago & Joliet

Smoke Test Report



Defect	Basin 6		Basin 7		Basin 8		Basin 9		Total		Repair Estimate
	No.	GPM	No.	GPM	No.	GPM	No.	GPM	No.	GPM	
1 Storm Sewer Connection	2	25.3	13	102.6	15	181.4	18	196.6	48	505.9	\$ 146,500
2 "Easy" System Repairs	16	31.7	25	51.5	81	152.8	24	44.6	146	280.6	\$ 127,950
3 "Easy" Private Repairs	40	73.0	68	111.9	165	446.8	46	82.5	319	714.2	\$ 37,050
4 Clean and Televis Sewer between MHs	27		43		136		45		95,380	* LF	\$ 476,900 *
5 Point Repairs	13	19.5	13	19.5	35	52.5	8	12.0	69	103.5	unknown
6 Complicated Private Repairs	26	9.0	51	37.6	151	103.3	45	18.4	273	168.3	\$1,197,450

158.5 323.1 936.8 354.1

- 1 Public inflow sources should be a priority to remove
- 2 Manhole lid replacements and frame adjustments are effective and can typically be completed as force account work
- 3 Downspout connections, sump pump connections are large inflow contributors, ordinance violations, and easily/inexpensively completed
- 4 Cleaning and televising should be part of a continuing maintenance program, once condition of the sewers has been assessed create a repair program
- * Based upon an average pipe diameter of 8 inches, average length of 380 feet between manholes, light cleaning for an average cost of \$5.00/LF
- 5 Point repairs can be by liner or excavation and pipe replacement, advance televising would be beneficial - cost is variable and not provided
- 6 Footing tile connections, window well drains, yard drains, leaking service pipes, etc. are expensive and difficult to correct on private property

7, 9, 8 or 8, 9, 7, 6?

Defect	Basin 6		Basin 7		Basin 8		Basin 9		Total		Repair Estimate
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<div>158.5323.1936.8354.1</div>											
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7, 9, 8 or 8, 9, 7, 6?

- 1. Smoke Testing seldom finds all the system defects.**

2. The severity of mainline leaks are difficult to estimate from smoke tests. The testing found 13 defects in Basin 6 with an estimated flow of 19.5 gpm, 13 defects in Basin 7 with an estimated flow of 19.5 gpm, 35 defects in Basin 8 with an estimated flow of 52.5 gpm, and 8 defects in Basin 9 with an estimated flow of 12.0 gpm.

The number and severity of the defects needs to be confirmed with follow-up testing as recommended by Robinson.

3. Inflow source amounts can vary widely dependent upon the areas draining to them and the precipitation event. Of the 816 identified defects, 161 can be identified as inflow sources and they are estimated to account for 63% of the total I/I.

3. Inflow sources

Identified Inflow Source	Basin 6		Basin 7		Basin 8		Basin 9		Total	
	No.	GPM	No.	GPM	No.	GPM	No.	GPM	No.	GPM
Area Drain	0	0	3	7.7	1	1.5	3	5.4	7	14.6
Creek / Stream	1	10.0	0	0	0	0	0	0	1	10.0
Drainage Ditch	0	0	1	5.0	0	0	0	0	1	5.0
Downspouts	14	64.2	20	73.4	47	369.4	16	67.3	97	574.3
Driveway Drain	0	0	3	9.5	0	0	0	0	3	9.5
Stairwell Drain	1	0.5	1	0.5	10	5.0	3	1.5	15	7.5
Storm Inlet, Catchbasin	1	15.3	6	80.4	14	179.9	15	191.2	36	466.8
Window Well Drain	1	0.5	1	0.5	7	3.5	0	0	9	4.5
Total	18	90.5	35	177.0	79	559.3	37	265.4	169	1,092.2

3. Inflow sources

Identified Inflow Source	Basin 6		Basin 7		Basin 8		Basin 9		Total	
	No.	GPM	No.	GPM	No.	GPM	No.	GPM	No.	GPM
Area Drain	0	0	3	7.7	1	1.5	3	5.4	7	14.6
Creek / Stream	1	10.0	0	0	0	0	0	0	1	10.0
Drainage Ditch	0	0	1	5.0	0	0	0	0	1	5.0
Downspouts	14	64.2	20	73.4	47	369.4	16	67.3	97	574.3
Driveway Drain	0	0	3	9.5	0	0	0	0	3	9.5
Stairwell Drain	1	0.5	1	0.5	10	5.0	3	1.5	15	7.5
Storm Inlet, Catchbasin	1	15.3	6	80.4	14	179.9	15	191.2	36	466.8
Window Well Drain	1	0.5	1	0.5	7	3.5	0	0	9	4.5
Total	18	90.5	35	177.0	79	559.3	37	265.4	169	1,092.2

Recommended Next Steps (Robinson)

1. Fix manhole defects
2. Manhole inspections
3. Video inspection of sewers and repairs
4. Confirm inlet connections (#1)
5. Investigate creek connections (#2)
6. Private sector (Downspouts #1 a)

Recommended Future Steps (Robinson)

7 a. Lateral inspection and repair (144)

7 b. Foundation drain disconnection (14)

8. Internal building inspections

9. Private source disconnection

Thank you!

Questions?